

#### **Engineering**



## **Department of Civil & Mineral Engineering**

# Application of advanced laboratory experiments in understanding the cause of localized damage zones in Nuclear Waste Repository in Finland

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#### Talk summary in two parts:

#### Part 1:

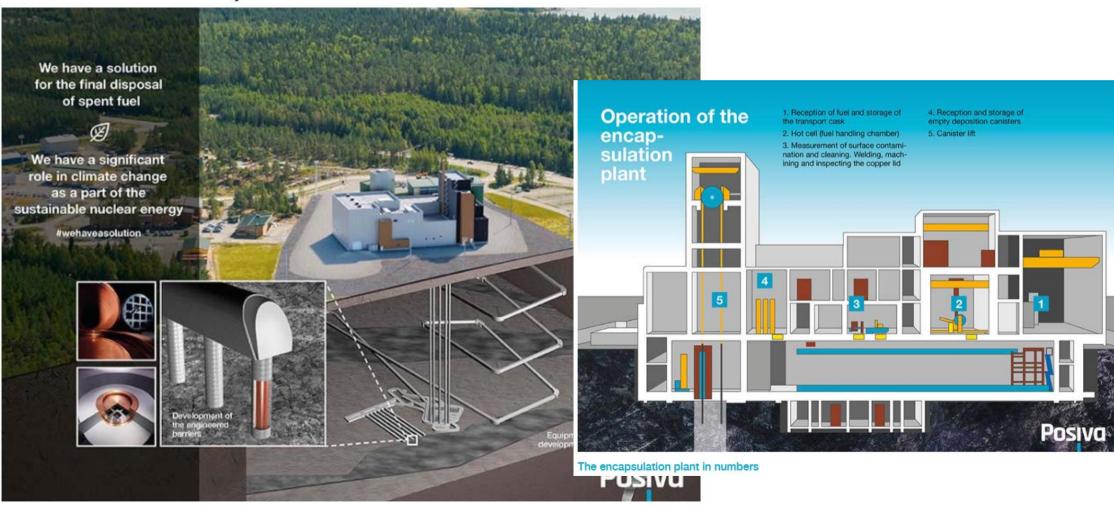
- Underground rock characterisation facility at ONKALO, Finland
- •Posiva's Olkiluoto Spalling Experiment (POSE) and its numerical modeling results/field observation/problems observed

#### Part 2:

- •Research objectives and our collaboration with Posiva Oy at U of T, the advantage of advanced research laboratory working at the interface of rock mechanics, rock physics and engineering seismology
- •Comparison of filed failure of the test holes with the results obtain using true-triaxial experiments at UofT
- •Conclusion

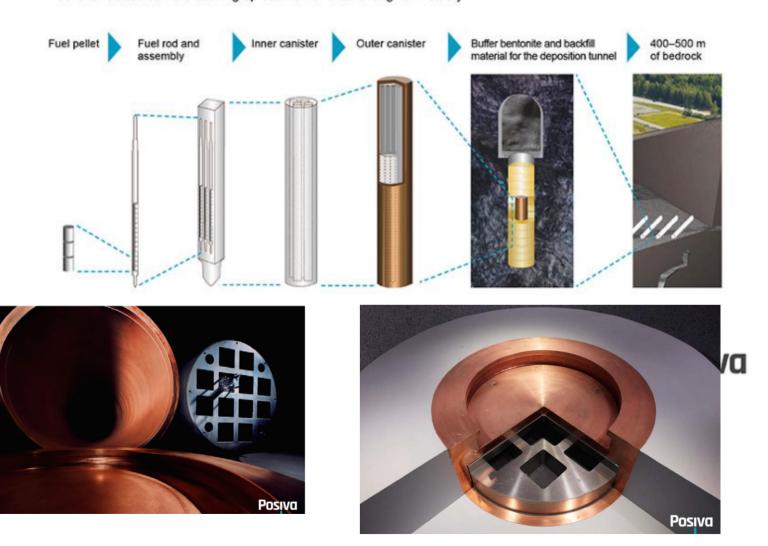
#### Final disposal

Finland is a pioneer of final disposal of spent fuel. No other country has yet reached the implementation phase of final disposal. Many countries using nuclear power have final disposal facilities for low- and medium-level waste, but final disposal of high-level spent nuclear fuel has not yet been launched anywhere.

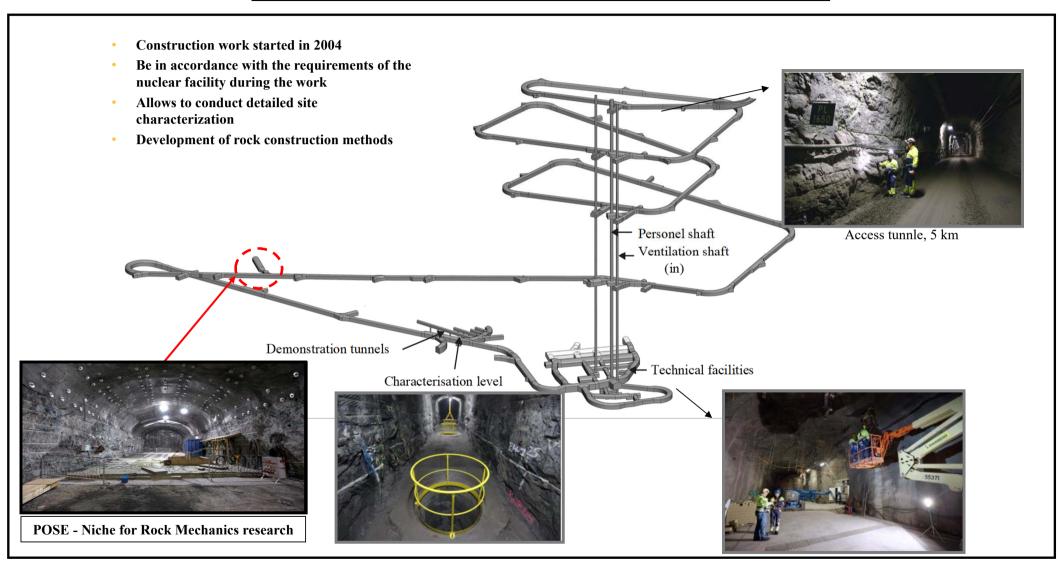


# **Only Safe Final Disposal Is Possible**

Multi-barrier principle of final disposal:
 Several release barriers backing up each other ensure long-term safety

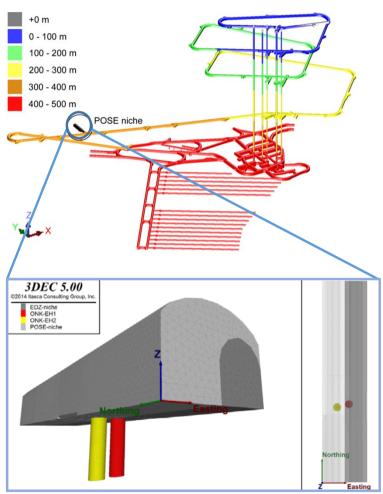


# **Underground Rock Characterisation Facility ONKALO**



M.H.B. Nasseri, M. Sehizadeh and R.P. Young, Posiva working report 2018-14, June 2018

## **Posiva's Olkiluoto Spalling Experiment (POSE)**



Model geometry with ONK-EH1 in red, ONK-EH2 in yellow. An inset on the right depicts the orientation of the experimental holes when viewed from above (Hakala & Valli, 2012).

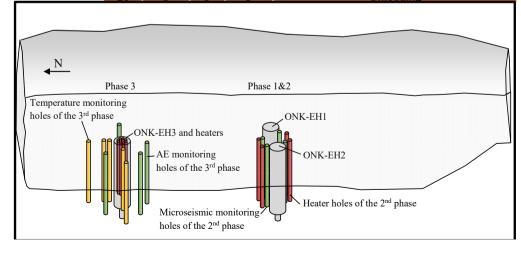
#### POSE Experiment Stress Path Determination

Modelling techniques and theoretical/experimental evaluation of 3D *in situ* stress variation during various stages of excavation at the site show that the three principal stresses change independently in the vicinity of the excavation.

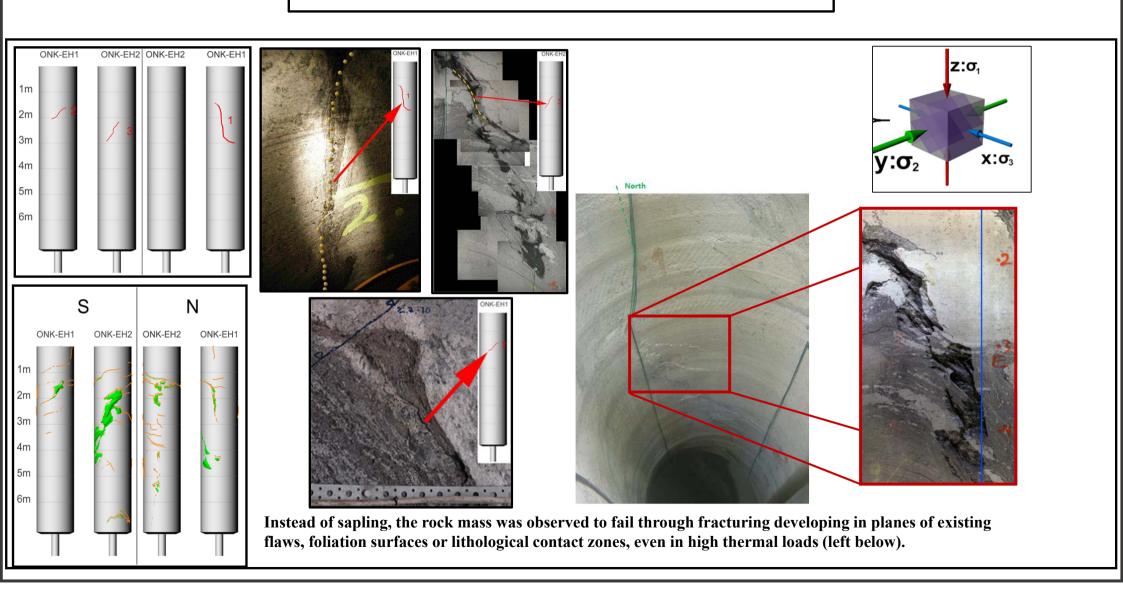
 $\sigma_2$ 

 $\sigma_1$ 

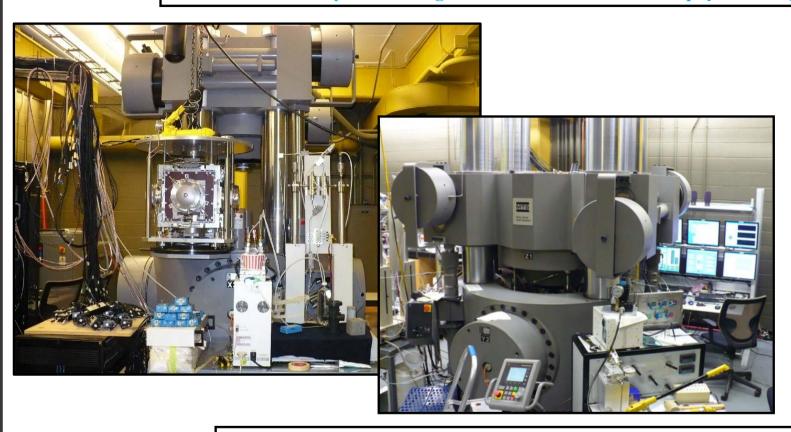
Steps	(MPa)	(MPa)	(MPa)	
0	5	5	5	Zero
1	10	10	10	Build of minimum principal stress $(\sigma_3)$
2	10	18	18	Build of intermediate principal stress $(\sigma_2)$
				Build major principal stress ( $\sigma_1$ ). In situ stress
3	10	18	27	state of $(\sigma_1, \sigma_2, \sigma_3)$ is reached
4	3	18	55	Excavation of niche and ONK-EH1
5	1	5	55	Excavation of ONK-EH2
6	1	10	85	2 weeks of heating
				Maximum temperature from POSE Phase 3
7	1	30	110	model
				Maximum temperature from Phase 1 & 2
8	1	20	140	model
9	10	18	27	Cooled to in situ
10	5	5	5	Unloading

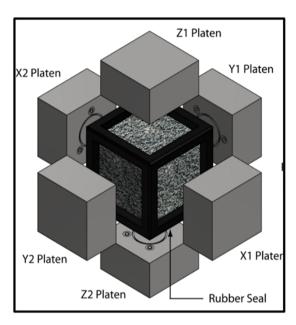


#### Rock mass response to excavation and thermal load



# MTS Poly axial testing frame with True-Triaxial Geophysical Imaging Cell



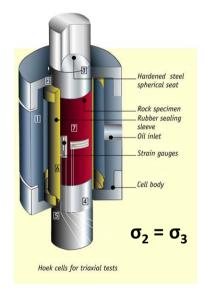


- $\bullet$ Polyaxial servo-controlled loading system; 6800 kN axial, 3400 kN lateral ,  $\sim$ 1GPa axial stress and 500 MPa lateral on a 8x8x8 cm cube,
- •Polyaxial (true triaxial) and triaxial geophysical imaging cells
- •Temp. up to 200°C
- •Full waveform continuous Acoustic Emission (18 sensor 3D array sampled at 10MHz up to 8hrs)
  - •3D velocity measurement system (including 6 compressional and 12shear sensors)
  - •Pore pressure control and 3D permeability along independently controlled axes

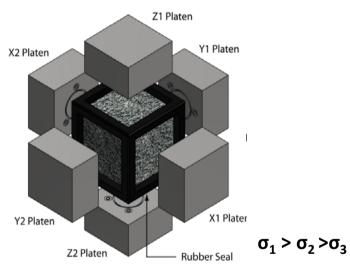
#### Research Motivation. We had to have a three dimensional testing approach!

- •The Posiva's Olkiluoto Spalling Experiment (POSE) stress path is complex and results in strength and behavioural characteristics that are not replicated with conventional triaxial testing where  $\sigma_2 = \sigma_3$
- •Construction of the planned deep geological repository at the Olkiluoto site involves isotropic and anisotropic rocks of different strength and deformational properties making 3D assessment of changing stresses a complex undertaking.

Henceforth, the aim of this research investigation is to replicate the POSE - Experiment in controlled laboratory conditions, in which the response of the prevailing rock types along with the effect of anisotropy could be understood further in detail using a unique true-triaxial geophysical imaging cell in which  $\sigma_1 > \sigma_2 > \sigma_3$ .

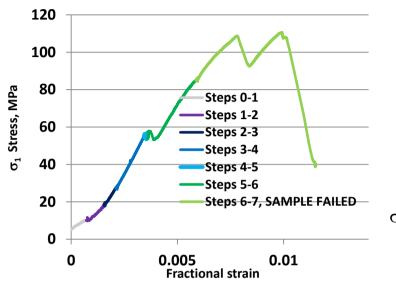


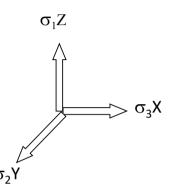
Conventional triaxial testing system

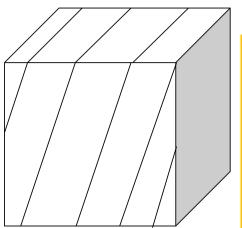


True triaxial testing system

#### 30° specimen, strain-strain, first round

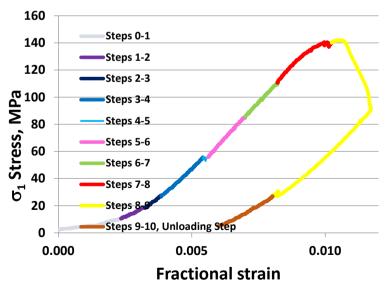


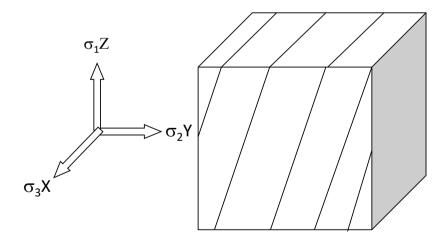




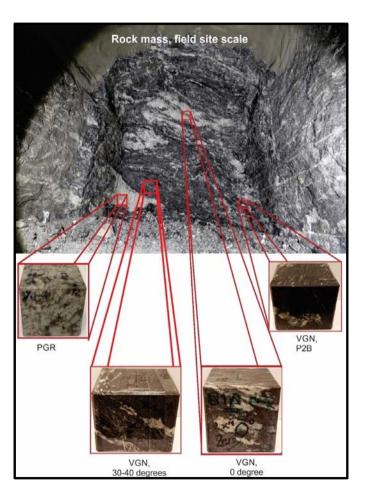
#### **Loading steps for the TTT experiments**

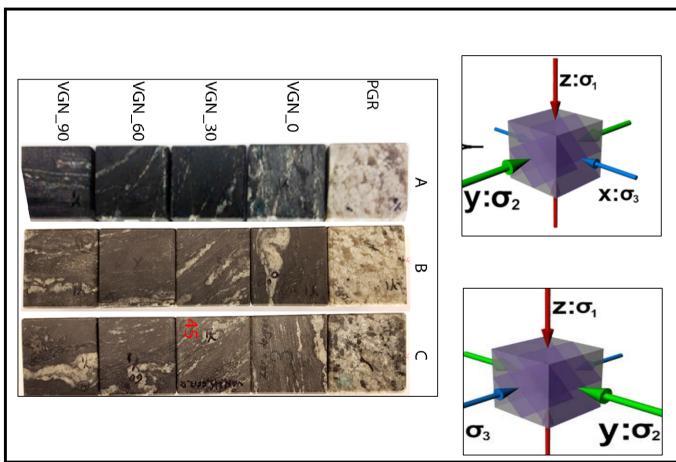
	s3	s2	s1	
Step	MPa	MPa	MPa	
(	0	0	0	zero
:	1 10	10	10	build s3
2	2 10	18	18	build s2
3	10	18	27	build s1 = In situ
4	1 3	18	55	Exca_test hole 1
į	5 1	. 5	55	Exca_test hole 2
(	5 1	10	85	heating 2 weeks
7	7 1	30	110	heating EH3 max
8	3 1	20	140	heating HE3 max
٩	10	18	27	cooled to in situ
10	0	0	0	zero





# Effect of anisotropy on strength, deformational and seismicity under TTTE Underground research laboratory in ONKALO, Finland

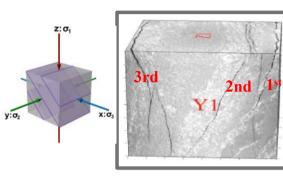




M.H.B. Nasseri, M. Sehizadeh and R.P. Young, Posiva working report 2018-14, June 2018

# Recognition of sequence of fracturing, using AE events and thin section analysis, 30° specimen

	s3	s2	s1	
Step	MPa	MPa	MPa	
0	0	0	0	zero
1	10	10	10	build s3
2	10	18	18	build s2
3	10	18	27	build s1 = In situ
4	3	18	55	Exca_test hole 1
5	1	5	55	Exca_test hole 2
6	1	10	85	heating 2 weeks
7	1	30	110	heating EH3 max
8	1	20	140	heating HE3 max
9	10	18	27	cooled to in situ
10	0	0	0	zero
6 7 8 9	1 1 1 10	10 30 20	85 110 140 27	heating 2 weeks heating EH3 max heating HE3 max cooled to in situ

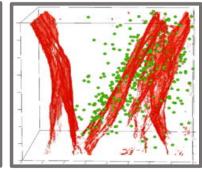


3D X-ray Micro-CT image

First episode of AE

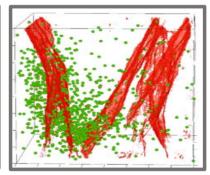
 $\sigma_1 = 55$ ,  $\sigma_2 = 5$ ,  $\sigma_3 = 1$ MPa Beginning of steps 5

Fracture propagation and second episode of AE

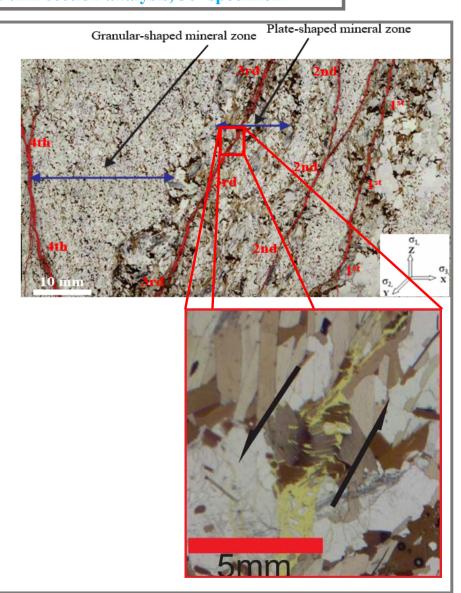


 $\sigma_1$ = 109,  $\sigma_2$ = 28,  $\sigma_3$ = 1MPa Middle of step 7

Third AE activity and failure



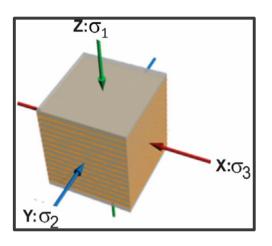
 $\sigma_1$ = 111,  $\sigma_2$ = 18,  $\sigma_3$ = 1MPa End of step 7



# 90° specimen

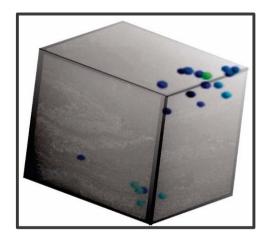
#### **Loading steps for the TTT experiments**

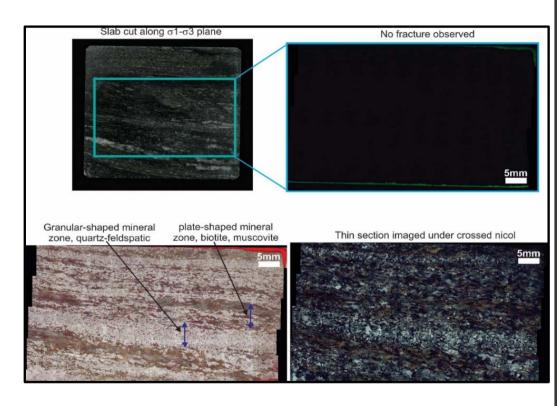
	s3	s2	s1	
Step	-	T —	MPa	
0	0	0	0	zero
1	10	10	10	build s3
2	10	18	18	build s2
3	10	18	27	build s1 = In situ
4	3	18	55	Exca_test hole 1
5	1	5	55	Exca_test hole 2
6	1	10	85	heating 2 weeks
7	1	30	110	heating EH3 max
8	1	20	140	heating HE3 max
9	10	18	27	cooled to in situ
10	0	0	0	zero





**Tested specimen** 



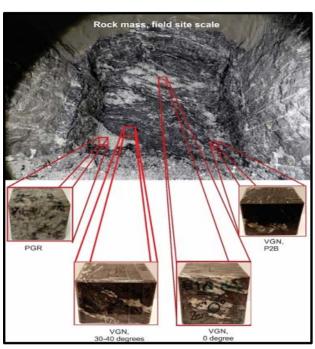


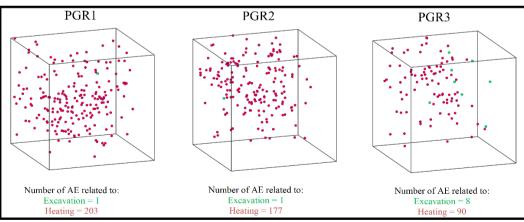
AE response End of step 9

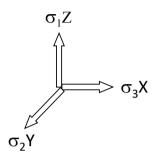
52<sup>nd</sup> US Rock Mechanics /Geomechanics Symposium

Westin Seattle – June 17-20, 2018

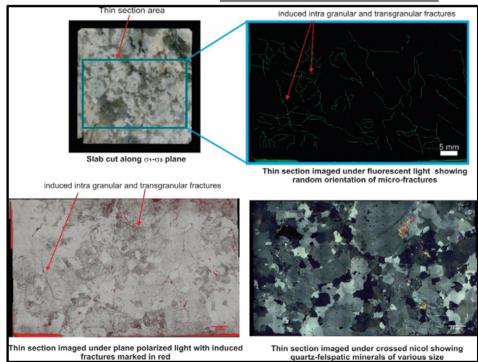
# Opening and closure of pre-existing fractures, using AE events and thin section analysis, pegmatite specimen



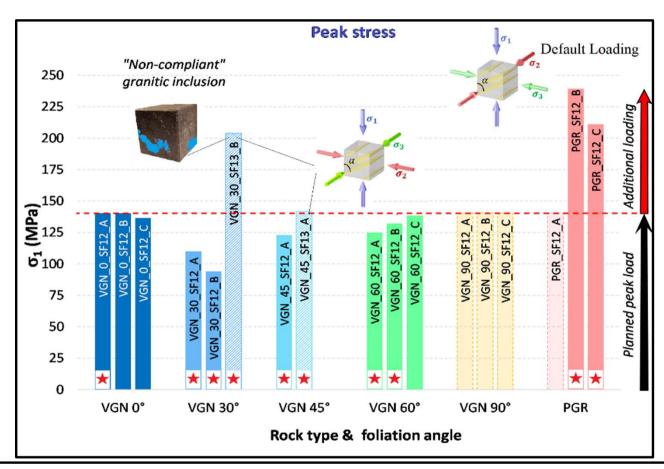








#### Results for all 17 tested specimens



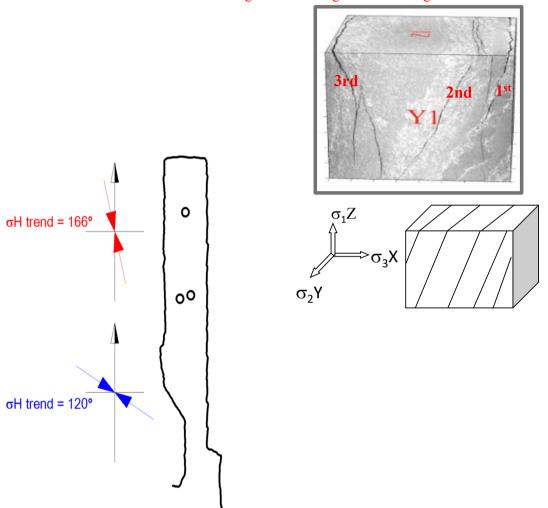
The peak stress results for VGN and PGR, with VGN results plotted according to the foliation determined from the specimens a priori. The hatch pattern indicates specimens that were tested with  $\sigma_2$  perpendicular to foliation. Specimens that were loaded over the planned peak load exceed the dashed red line. Red Stars indicate samples that failed.

#### **Conclusion**

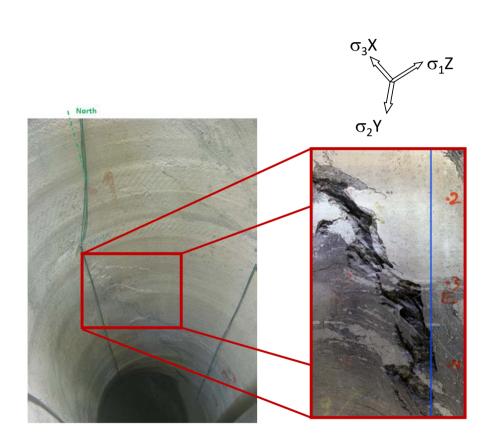
- The 30° micaceous gneissic specimens show the lowest strength and start the initiation of fracturing as early as loading in step 5 i.e. during the excavation of the second test hole.
- 90° gneissic specimen did not fail and looked intact after the test and showed little AE activity at the end of loading step 8.
- AE activity highlights the failure plane in the tested specimens 0°, 30° and 60°.
- Spatial and temporal AE events studies shows that the failure pattern in gneissic specimen is localized along foliation planes (0°, 30° and 60°) in comparison to pegmatite specimen which does not show localized damages.
- We conclude that where ever along the length of the test holes the induced in-situ main principal direction stress exceeds 50 MPa and makes an angle of about 30° with micaceous gneiss rock type or at the boundary with pegmatite fracture initiation is triggered and leads to full failure during the heating and cooling stages in the second test hole.
- Based on the true-triaxial test results, it became easier to understand the cause of localized excavation damages in an anisotropic rock mass selected for nuclear waste repository site in Finland.
  - Acknowledgment: My special appreciation to Mr. Laszlo Lombos for their continuous support with many industrial projects during my 25 years of research career at University of Toronto.
  - Special thanks to:
  - R. M. Sehizadeh,
  - Dr. Maria Tibbo
  - · Dr. Willian Feng
  - · Dr. Sherveen

# Failure of test holes after heating stages

MRI image of the 30 degree micaceous gneiss tested in the laboratory



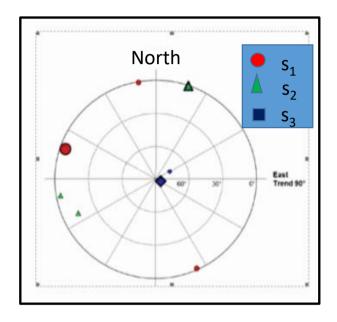
The trend of the maximum principal stress according to two interpretations: the EDZ/access tunnel trend in red and the ONK-EH3 trend in blue (Hakala & Valli, 2013).

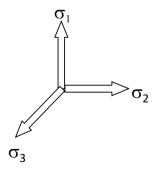


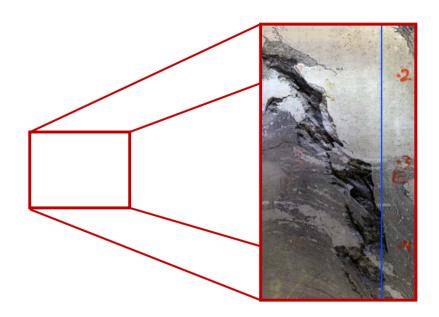
A close-up of heating induced damage in ONK-EH2 at a depth level of  $2-5\,\mathrm{m}$  from the top of the hole. The pillar centre line is indicated by a blue line (Johansson et al. 2013).

The Deposition Hole Test Results and Observation: Effect of Changes in Main Principal Stress Magnitude and orientations with Respect to weak planes

Our field visit and Examination of selective failure of heating test holes







A close-up of heating induced damage in ONK-EH2 at a depth level of 2-5 m from the top of the hole. The pillar centre line is indicated by a blue line (Johansson et al. 2013).

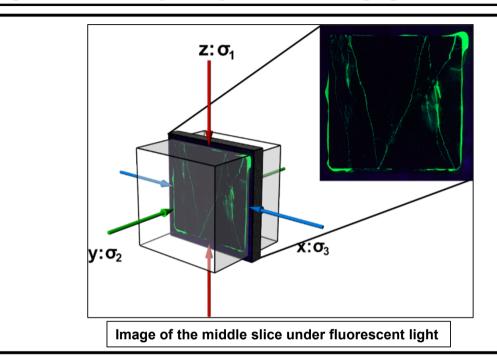
# Specimen stabilizing, slicing and thin section preparation

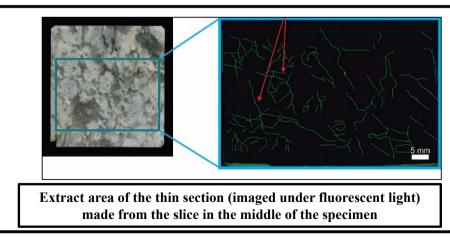


0° Mica gneiss



30° Mica gneiss







60° Mica gneiss



90° Mica gneiss